

ModWoodLife

#### COST Action FP1407 2<sup>nd</sup> Conference

*"Innovative production technologies and increased wood products recycling and reuse"* **Brno, Czech Republic** 29 – 30th September 2016

### ELASTIC PROPERTIES OF THERMO-HYDRO-MECHANICALLY MODIFIED BAMBOO (Guadua angustifolia Kunth) MEASURED IN TENSION

### **Dr Hector F. Archila**

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- **1.** Why densification and engineered bamboo?
- 2. Research @ Bath
- 3. Results & Discussion
- 4. Conclusions



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## **Densification**

### - Thermo-hydro-mechanical (THM) modifications -

Chemical modification	Etherification, Olification, Acetylation, Furfurylation, Formaldehydation, etc.		
	Wood drying		
Thermo-Hydro (TH) modifications	Wood ageing		
	Heat treatment	Thermowood	
		Plato Wood	
		Retification & Perdure	
		Oil Heat Treatment (OHT)	
	Percentituted wood	Composites	
		Fibre webs	
	Neconstituted wood	Veneer processing	
		Biomass processing	
	Ponding	Solid wood	
	Bending	Laminated wood	
Thermo-Hydro-Mechanical (THM) modifications	Wood chaping	Surface	
		Cross-sectional	
	THM densification	Open system (TM & THM)	
	n five densitied for	Closed system (THM)	
	Wood welding		

*Heat, pressure and water interact to achieve:* 

- ✓ Changes in shape (e.g. bending)
- ✓ Improve weather resistance.
- ✓ Reduce fungi decay and hygrospicity.
- Increase in density, elastic properties & hardness (by reducing void cellular spaces – cond. vessels, parenchyma) without cell damage (Sandberg & Kutnar, 2014).









# **THM modifications**

#### - Wood and bamboo -

Round log of wood prior to shaping



Squared log post-shaping.



Wood shaping Photograph by Morsing (2000).

Bending round bamboo with a gas flame burner and green Guadua by manual force (www.guaduabamboo.com)





#### **Bamboo bending**

Bent Guadua construction temporary built in Pereira by Simon Velez.









**Bamboo shaping (nosing)** (Kitazawa et al., 2004)







**Open TM** Densification (Li et al., 1994).



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### Engineered Bamboo -full potential of the plant-



- ✓ Long lasting Value Added products
- ✓ Exploit mechanical properties
- ✓ Reduce labour, transport and wastage (30-50%)
- ✓ Overcome natural defects & shape irregularities
- ✓ Standardisable & lower CO2 emissions products.







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Source BothBest Enterprise Co., Ltd



 ✓ Carbon Footprint over life cycle (kgCO2eq /m<sup>3</sup>)





Carbon Footprint over life cycle (kgCO2eq / m3 building material) for various common building materials (this report, Idemat 2014 database and Vogtländer et al. 2014). Source: Inbar Technical report No. 35, 2014 by Vogtländer & van der Lugt Sharma, B., Gatóo, A., Bock, M. and Ramage, M., 2015. Engineered bamboo for structural applications. Construction and Building Materials, 81, pp. 66-73.







100

Technical

ceramics

Metal ferrous and non-ferrous



# Research @ U-Bath

- ✓ THM Densification
- Mechanical testing (densified bamboo)
- ✓ Results and discussion



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50% wastage Loss of stiffness Labour & Energy intensive

### **Solution**

Less wastage (20%) Improved stiffness (2x\*) Straight-forward processing





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# **Densification Technology**

-full potential of the plant-



### **Materials**

	Samples A Raw un-processed	Samples B Hot pressed + Dried	Samples C Pre-soaked + Hot pressed + Dried
Time	0	20 min	20 min
Pressure	0	60 kg/cm <sup>2</sup>	60 kg/cm <sup>2</sup>
Temperature	0	150° C	150° C
Compress. Set (C)	0	46.08%	42.51%
Oven dried density	0.54 g/cm <sup>3</sup>	0.81 g/cm <sup>3</sup>	0.89 g/cm <sup>3</sup>
		b)	









## **Image-J Analysis**

### Fibre's surface area



25.53 %

45.57 %

47.78 %



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# **Microscopy and Image analysis**



#### Stereo microscope images

(a) before densification and (b) following THMT.



#### **Digitally contrasted pictures** from two inverted reflected-light microscope images of the same region before (a) and after (b) densification.



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## **Longitudinal Tensile test**

### Elastic parameters MOE and V of small clear specimens

- ✓ Samples conditioned at 27±2° C and RH of 70±5% for 20 days (MC=12%).
- ✓ Load limited to elastic deformation.
- ✓ Rate 0.5mm/min (all tests)
- ✓ Strain gauges, resistance 350 ohms.
- ✓ Four specimens per sample.
- ✓ (BS 373 / BS EN 408)







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Figure 7-4. Typical initial load-strain graph for samples A, B and C under longitudinal tensile test. The portion of the graph between 0.1  $F_{max}$  and 0.4  $F_{max}$  was used for the linear regression analyses and values within this portion and a correlation coefficient R<sup>2</sup>  $\geq$  0.99 were used for calculating  $v_{13}$ , E<sub>12</sub> and  $v_{12}$  for all samples.

#### Material characterization

Elastic stress versus strain plots for longitudinal loading

$$E_t = \frac{(F_2 - F_1)}{(u_2 - u_1) \cdot A}$$

is the increase of load between 0.1  $F_{max}$  and 0.4  $F_{max}$  $\cdot u_1$ ) is the increase of deformation corresponding to  $(F_2 - F_1)$  using the linear regression line.

$$v_{12} = -\frac{\varepsilon_2}{\varepsilon_1}$$

Poisson's ratio in the 1-2 plane caused by a load applied along X1.

$$\upsilon_{13} = -\frac{\varepsilon_3}{\varepsilon_1}$$

Poisson's ratio in the 1-3 plane caused by a load applied along X1.









#### Material characterization

BY

Box plot of the Young's modulus longitudinal to the direction of the fibres ( $E_1$ ) of samples A, B and C.



**FP**140

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#### Poisson's ratios v<sub>12</sub> vs density

Poisson's ratios ratio of passive strain along the tangential direction  $(X_2)$  to active strain along the longitudinal direction  $(X_1)$ .

#### Poisson's ratios v<sub>13</sub> vs density

Poisson's ratios ratio of passive strain along the radial direction  $(X_3)$  to active strain along the longitudinal direction  $(X_1)$ .

Figure 7-7. Box plots of the Poisson's ratios  $v_{12}$  (a) and  $v_{13}$  (b) of samples A, B and C.









# **Testing of densified strips**

#### Elastic parameters MOE, G and V of individual layers

Property	Pre-THM (Control)		Post-THM (densified)	
E <sub>L (Tension)</sub>	16.88 ± 0.33 GPa	22.80 ± 0.73 Gpa	<b>30.72</b> ± 0.43 GPa	~ 2x
V <sub>LT</sub>	$0.28 \pm 0.01$	$0.33 \pm 0.01$	$0.26 \pm 0.04$	
CoV	3%	4%	2%	
V <sub>LR</sub>	$\textbf{0.30} \pm 0.02$	$\textbf{0.33} \pm 0.02$	0.09 ± 0.02	
CoV	5%	5%	18%	
Compression set (C)	0 %	46.08 %	42.51 %	~ 2x
Density (ρ)	543.3 kg/m <sup>3</sup>	814.6 kg/m <sup>3</sup>	890.9 kg/m <sup>3</sup>	
Fibre surface	25.53%	45.57%	47.78%	
Specific stiffness (av.)	31.06 m <sup>2</sup> s <sup>-2</sup>	27.99 m <sup>2</sup> s <sup>-2</sup>	34.49 m <sup>2</sup> s <sup>-2</sup>	



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# Conclusions

- ✓ THM modifications increased the physical and mechanical properties of bamboo Guadua with minimal damage to the cell structure.
- Values of mechanical properties of non-treated samples of Guadua (Sample A) obtained from the testing programme are in accord with those reported in the literature (García et al., 2012; Ghavami & Marinho, 2005; Rusinque & Takeuchi-Tam, 2007; Takeuchi-Tam & González, 2007).
- ✓ The coefficient of variation (CoV) for the obtained results is in the range of variation for the mechanical properties of clear wood; as defined by FPL (2010) CoV for the modulus of elasticity measured in tension parallel to the grain is 25%.
- ν<sub>21</sub> and v<sub>31</sub> are very small and less precisely determined due to their high scattering of results; research conducted by Ling et al . (2009) on the determination of the **Poison's ratios of wood in compression** found similar issues.









### Děkuji, Thanks,...! & Questions...?

### Hector F. Archila

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## **Structural engineered bamboo**

### Manufacture of cross laminated Guadua panels (G-XLam)



Epoxy resin content ~4% (Sicomin SR 5550) @ Spreading rate =  $215 \text{ g/m}^2$ 

✓ Cold pressed ~ 35 kg/cm<sup>2</sup> \*

G-XLam 5  $t = 27.5 \pm 1.5 \text{ mm}$  (~1 inch) G-XLam 3



 $t = 16.5 \pm 0.9 \text{ mm}$  (~0.6 inch)



**G-XLam panels** (3-5 layers) (max 1,20 x 1,20 m)\*\*













# **Testing of G-XLam panels**

### Elastic parameters MOE, G, V



1 & 2. High speed cameras; 3 G-XLam panel; 4. Test machine; 5. Monitor

### In-plane compression test using digital image correlation (DIC) (BS EN 789)



#### Picture frame panel shear test using DIC (ASTM E5 19-02)

- ✓ 600x600mm G-XLam3-5 panels
- ✓ Panels conditioned at 27±2° C and RH of 70±5% for 20 days (MC=12%).
- $\checkmark$  Load limited to elastic deformation.
- ✓ Rate 0.5mm/min (all tests)
- ✓ LVDT and virtual extensometers.



#### Bending test (BS EN 789)



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# **Testing of G-XLam panels**

#### Elastic parameters MOE, G and bending strength

	Birch plywood (8.5 to 25 mm)	CLT-3	CLT-5	G-XLam 3 (~ 17 mm) <sub>Mech. Test</sub>	G-XLam 5 (~ 27 mm) <sub>Mech. Test</sub>
E <sub>L (0)</sub>	9.0 GPa	7.42 GPa	6.74 GPa	14.86 GPa	12.48 GPa
Е <sub>т (90)</sub>	7.9 GPa	3.91 GPa	4.62 GPa	7.43 GPa	8.74 GPa
G <sub>v (xy)</sub>	-	0.65 GPa	-	0.67 GPa	0.72 GPa
E <sub>m (bend)</sub>	9.7 GPa	11.6 GPa	-	23.68 GPa	19.36 GPa
$f_{ m m\ (bend)}$	_	-	-	99.92 GPa	91.76 GPa

✓ CoV < 12% (coefficient of variation)



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### **Engineered Bamboo** -for structural applications-





Figure 7-9. Typical specimen size and compression test set up for load applied along the radial direction.



Figure 7-10. Typical specimen size and compression test set up for load applied along the tangential direction.



Figure 7-11. Typical strain-stress graph plotted from the results of a radial compression test on specimens of sample C (scan sessions represent the repetitions undertaken).

Elastic values and Poisson's ratios of samples A, B and C were calculated using the strain values  $(u_2 - u_1)$  on the linear regression line between  $0.1F_{max}$  and  $0.4F_{max}$ . Calculation of the compression modulus of elasticity  $(E_c)$  followed the formula:

$$E_c = \frac{(F_2 - F_1)}{(u_2 - u_1) \cdot A}$$
 7-4

where  $(F_2 - F_1)$  is the increase of load between  $0.1F_{max}$  and  $0.4F_{max}$  and  $0.4F_{max}$  and  $(u_2 - u_1)$  is the increase of deformation corresponding to  $(F_2 - F_1)$  using the linear regression line.



Figure 7-20. a) losipescu shear test set-up on INSTRON. b) Specifications of the strain gauge used. c) Typical load-strain graph for +45 and -45 strain gauges readings on front and back faces and their average for both orientations (+45 and -45).

where  $\Delta \tau \mbox{ is the difference of shear stress between } \tau_2 \mbox{ and } \tau_1 \mbox{ and } \Delta \Upsilon \mbox{ is the difference of shear strain between } \Upsilon_2 \mbox{ and } \Upsilon_1. \label{eq:constraint}$ 

 $G_{12}^{chord} = \frac{(\Delta \tau)}{(\Delta \Upsilon)}$ 

7-6

G-XLam3 and G-XLam5 panels with dimensions of 600x600mm and 700x700mm were subjected to mechanical testing. These sizes are considered to be representative volume elements (RVE) of potentially larger commercial size panels. The testing programme included in-plane compression tests in the X1 (longitudinal) and X2 (transverse) directions, four point bending test and in-plane shear test (Figure 10-1).



Figure 10-1. a) Geometric (X1, X2, X3) axes of G-XLam3 and G-XLam5 panels. b) Diagram of the compression test in the longitudinal direction of the panel. c) Diagram of the compression test in the transverse direction of the panel. d) Diagram illustrating 4-point bending test setup for G-XLam3 and G-XLam5 panels. e) Diagram of the picture frame panel shear test.

Engineering strain ( $\epsilon$ ) was then calculated as the change in length  $\Delta L$  per unit of original length L, as expressed in Equation 10-2.

> $\mathcal{E} = \frac{(\Delta L)}{(L)} = \frac{(l_1 - l_0)}{(l_0)}$ 10-2

 $l_0$  is the initial length of the extension eter and  $l_1$  its final length.

$$Ep_{C,0,90} = \frac{(F_2 - F_1)l}{(u_2 - u_1)A}$$
 10-3

 $F_2 - F_1$  is the increment of load between  $0.1F_{max}$  and  $0.4F_{max}$  $u_2 - u_1$  is the increment of engineering strain corresponding to  $F_2 - F_1$ l is the gauge length (A-B length of the virtual extensometer), and

$$\gamma_{xy} = \frac{\varepsilon_x + \varepsilon_y}{2} \left( \tan\theta' + \frac{1}{\tan\theta'} \right); \qquad \tau_{xy} = \frac{0.707 P}{l t}; \qquad G_{xy} = \frac{\tau_{xy}}{\gamma_{xy}} \qquad 10-4$$

Ex and Ey are the measured strains in x and y P is the force applied, l the side-length of the specimen and t the thickness of the panel.

$$E \cdot I = \frac{23 \cdot L^3}{1296} \cdot \frac{\Delta F}{\Delta \delta_{centre}} \quad Ep_{m, \ global} = \frac{23 \cdot L^3}{1296 \cdot I} \cdot \frac{(f_2 - f_1)}{(w_2 - w_1)}$$
 10-5

 $E \cdot I$  is the bending stiffness,

L is the span,

 $f_2 - f_1$  is the increment of load between  $0.1F_{max}$  and  $0.4F_{max}$ 

 $w_2 - w_1$  is the increment of deflection between  $f_2 - f_1$  and

I is the moment of inertia or second moment of area of the beam section ( $I = b \cdot d^3/12$ ).

## **Numerical and FE analysis**

#### Prediction and simulation = Standardization



$$Ep_{t,c} = \frac{\sum_{i=1}^{n-1} E_{i\,t,c} \cdot h_i}{\sum_{i=1}^{n-1} h_i}$$

 ✓ Analytical design methods for plywood and CLT panels
 BS EN 14272 (BSI 2011)

✓ BS EN 789:2004 [8] standard for structural timber elements







Materials & Features

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